PROPOSED AMENITIES BUILDING ISAAC MOORE PARK, KENILWORTH SUNSHINE COAST REGIONAL COUNCIL STRUCTURAL DRAWINGS

RPD
LOT 10
RP 860722

SCHEDULE OF STRUCTURAL DRAWINGS

DWG	DESCRIPTION
5576 03 S 100	SITE PLAN & DRAWING SCHEDULE
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5576 03 S 421	WALL FRAMING DETAILS
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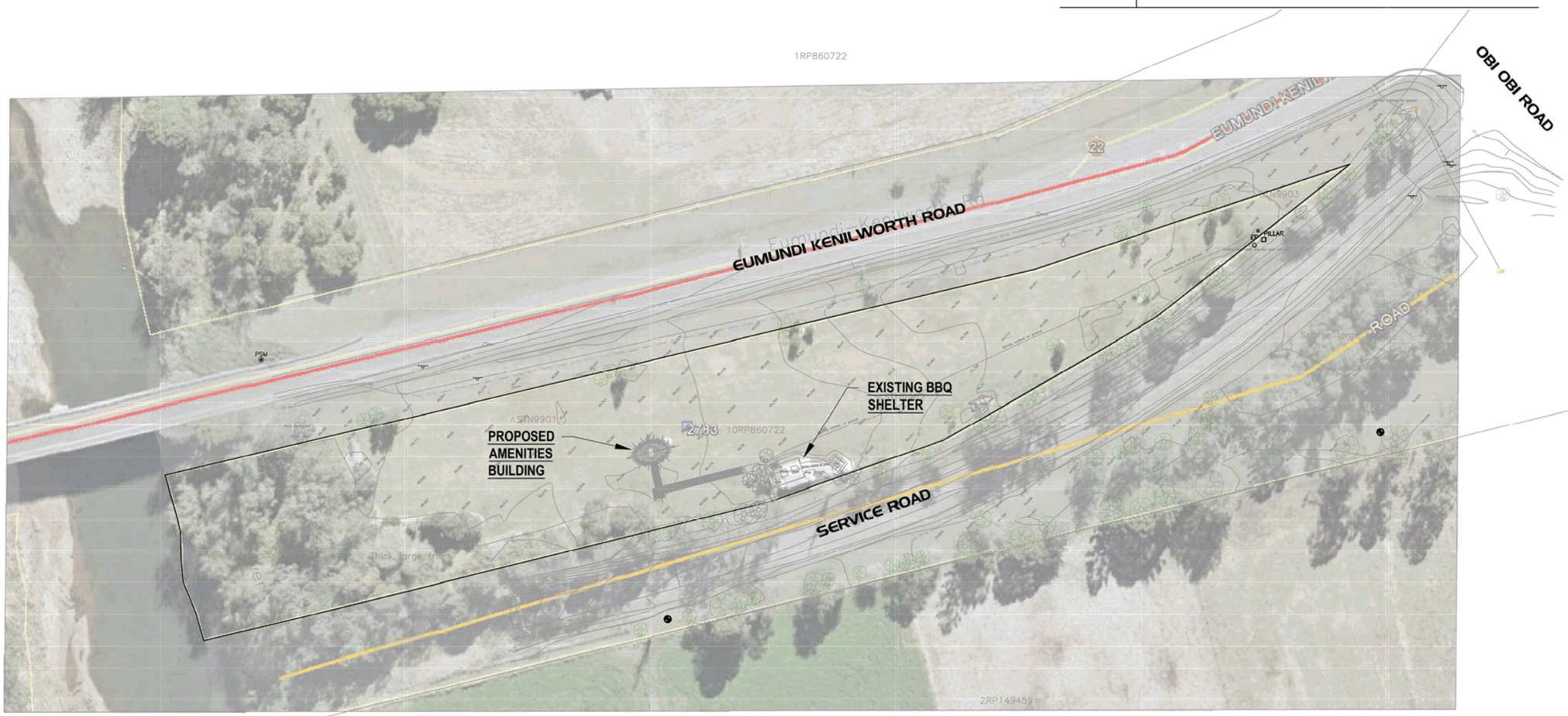
5576 03 S 510	WALKWAY FRAMING DETAILS



SITE PLAN
SCALE 1:1000

CONTOUR
A.B.N. 62 117 061 659
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PLAN DETAILS:
Designed: SD
Drawn: BS
Checked: RM
Datum: AHD
Date: JAN 2019

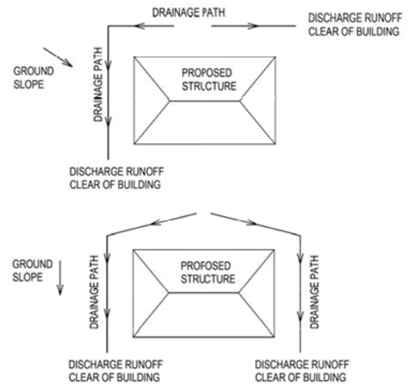
PLAN AMENDMENTS:

Rev	Date	Comment	Altered	Checked

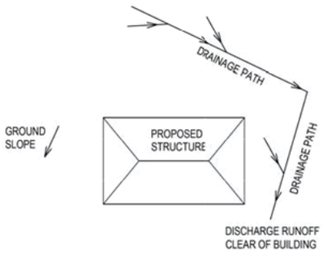
PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
SITE PLAN
SCHEDULE OF DRAWINGS
PRELIMINARY ISSUE FOR REPORT ONLY

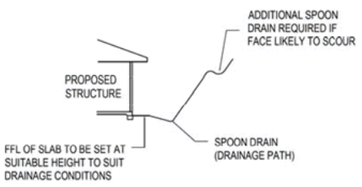
PLAN No. 5576 03 S 100 REVISION



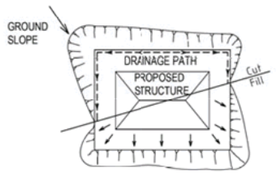
TYPICAL SITE DRAINAGE LAYOUT



DRAINAGE LAYOUT FOR WET SITES



SURFACE DRAINS ALONG LINE OF SLOPES



DRAINAGE LAYOUT FOR CUT AND FILL SITES

NOTE:
ENSURE SITE DRAINAGE IS MAIN"AINED DURING AND POST CONSTRUCTION

GENERAL NOTES:

1. These drawings shall be read in conjunction with all other consultants drawings and specifications.
2. Before proceeding with the works any discrepancies in the engineering drawings shall be referred for decision to the Engineer.
3. Setting out dimensions and sizes of members or elements shall not be obtained by scaling the drawings.
4. All dimensions shown on the engineering drawings shall be checked by the contractor before commencement of fabrication or site works.
5. During construction, the structure shall be maintained in a stable condition. Construction loads must not exceed the safe load capacity of the structure at any time during construction. If in doubt ask the Engineer.
6. All workmanship and materials shall be in accordance the requirements of the current editions including Amendments of the relevant SAA Codes of Practice, except as varied by the Contract Documents and of the By-Laws of the Local Government Authority.
7. Wind loads for this project have been determined in accordance with AS1170.2 - 2002
   Site specific design wind speed factors adopted for this project are as follows:

Importance Level		= 2
Region		= B
Basic Regional Wind Speeds	V500(u)	= 57m/s (ultimate)
	V100 (p)	= 48m/s (permissible)
	V20 (s)	= 38m/s (service)
Direction multiplier	Md	= 0.95
Terrain / Height multiplier	Mz,cat	= 0.95
Shielding multiplier	Ms	= 1.00
Topographic multiplier	Mt	= 1.00
Internal pressure coefficient	Cpi	= +0.0, -0.3
Limit State Design Wind Speed	Vdes	= 47.1 m/s (N3)

8. Permanent, imposed and other loads (Dead & Live) for this project have been determined in accordance with AS1170.1 - 2002 for the following loadings:
   DEAD LOADS
   Floors, self weight plus 0.5kPa distributed, gen 1.0Kpa to tiled areas
   Roofs, self weight only.
   LIVE LOADS
   External Floors, 4.0kPa distributed, 2.7kN concentrated.
   SOIL LOADS

Soil Density	= N/A
Pressure Coefficients	= N/A

   EARTHQUAKE

Importance Level	= 2
Annual probability of exceedance	P = 1/500
Probability factor	Kp = 1.0
Hazard factor	Z = 0.07
Site soil class	= Ce
Structural building height	Hn = <12m
Earthquake design category	= I

GEOTECHNICAL

A sub-soil investigations was undertaken at this site by Tectonic Geotechnical Soil Testing report No. 17119-001-Rev0 this site has been classified as class H2 in accordance with AS2870 - 2011.
The Building Works contractor must review this report and obtain confirmation from a geotechnical engineer that the report is representative after site earthworks have been completed.
Refer to this report for details regarding:

- Temporary batter slopes and positive support options for proposed excavations.
- Water table levels and if De-watering of proposed excavations is required
- Site drainage recommendations

A general description of various soil classification materials is listed in the following table.

SITE CLASS	FOUNDATIONS
CLASS A	Mostly sand and rock sites with little or no ground movement from moisture changes.
CLASS S	Slightly reactive clay sites with only slight ground movement potential from moisture changes.
CLASS M	Moderately reactive clay or silt sites which can experience moderate ground movement potential from moisture changes.
CLASS H1-H2	Highly reactive clay sites, which can experience high ground movement from moisture changes.
CLASS E	Extremely reactive sites, which can experience extreme ground movement from moisture changes.
CLASS - P	Sites which include soft soils, such as soft clay or silt or loose sands; uncontrolled fill; landslip risk; mine subsidence; collapsing soils; soil subject to to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise.

EARTHWORKS NOTES:

Where any filling is carried out on the site after the completion of the site investigation and civil earth works, the following recommendation shall be followed:-
1. The site shall be stripped of all vegetation prior to the placement of any filling material.
2. Compaction Standards for filling material
   A. Controlled Filling
   (a) Sand filling up to 800 deep, placed in maximum 300 thick layers and compacted with a vibrating roller to achieve a minimum Dynamic Cone Penetrometer reading of 13 blows per 300 penetration (refer AS 1329.F3.3). Sand fill may be flooded if necessary to achieve this compaction standard, provided the water can subsequently be drained from the site.
   (b) Non-sand filling up to 400 deep, placed in maximum 150 thick layers and compacted with a mechanical roller. Clay fill shall be moist during compaction.
   (c) Controlled filling shall continue for at least 1 meter past the edge of the structure and shall be battered back to the original surface with a slope of no more than 1:2 (Vert : Hor).
   (d) Where the depth of filling material exceeds that specified above, the filling material shall be compacted as follows:-
   Sand fill to 70% Density Index in accordance with AS1329.E6.1
   Non-sand fill to 98% Maximum Dry Density (Standard Compaction) in accordance with AS1329 cl 5.4.1
   Compaction tests shall be carried out on each layer as the filling is placed and results provided to the Engineer for approval.

CONCRETE NOTES:

1. All workmanship and materials shall be in accordance with the requirements of the relevant current Australian Standard Codes (AS3600-2001 & AS2870-1996), Building By-Laws and the ordinances of the relevant Loca Authority.
2. Concrete Specification

Application	Conc Str (MPa)	Max Slump (mm)	Max Agg. Size (mm)	Min. Cover (mm)
Footings	N25	100 (+20, -10)	20	50
External Slab on ground	N25	80 (+20, -10)	20	40
Internal Slab on ground	N25	80 (+20, -10)	20	30
External Suspended slab	N32	80 (+20, -10)	20	40
Internal Suspended slab	N32	80 (+20, -10)	20	30
Columns and walls	N32	80 (+20, -10)	20	40
Pile caps	N25	80 (+20, -10)	20	50

   All concrete testing is to be carried out by a NATA registered laboratory, in strict accordance with the requirements of AS1012. The following minimum testing schedule is to be adopted in addition to the requirements of the manufacturer's quality assurance program.
   - A minimum of one test (3 cylinders) to be taken from each element cast on any given day (eg. Columns, footings, etc)
   - A minimum of one test (3 cylinders) to be taken from each concrete strength grade cast on any given day.
   - A minimum of one test (3 cylinders) to be taken from each 50m³ or part thereof cast on any given day for each strength grade.
   - All samples are to be taken from the agitator, on site, tested by a NATA registered laboratory and results provided to the Engineer for review
3. No penetrations, recesses or chases other than those shown on the structural drawings shall be made in concrete members without the approval of the Engineer.
4. Unless noted otherwise all formed edges and corners of concrete members shall have 20mm chamfers. Provide drip grooves to the perimeter of the soffits of all concrete members.

CONCRETE SLABS:

Preparation:
1. Subgrade to be proof rolled with any unsuitable material removed and replaced in accordance with earthworks notes.
2. Any filling under slab to be placed in accordance with earthworks notes.
3. Place and compact a 50mm layer of bedding sand under slab areas with 0.2mm vapor barrier over, tap and seal all joints.
3. At penetrations in slabs, unless otherwise detailed reinforcement must not be cut but shall be gathered equally to each side of the penetrations and extra reinforcement provided between the penetrations as directed by the Engineer.
4. During dry weather moisten timber formwork to assist curing of surfaces.

Placing:
1. Ensure no water is added to the concrete after initial batching.
2. No concrete to be poured when site temperature exceeds 35° C or falls below 5° C.
3. Constructions Joints or pour breaks where not shown on the approved plans or details shall be located and formed to the approval of the Engineer

Curing:
1. Curing by continuous wetting and/or covering for a period of not less than 7 days or by application of an appropriate sprayed membrane curing compound.
2. Retain form work as long as possible on vertical surfaces

Increased crack control methods:
1. Use of low shrink concrete with a design shrinkage strain less than 650x10-6
2. Concrete should not be placed when maximum daily temperatures are expected to rise over 30°C or when site wind speeds are expected to exceed 15 knots.

Tiling:
1. Where floor slabs are to be tiled, the following recommendations are made, in order to minimize the effects of shrinkage:-
   - Provision of SL92 slab mesh (or equivalent).
   - Placement of tiles should be delayed for as long as possible (preferably for at least 3 months)
   - Use of elastic tile adhesives and provision of flexible joints between tiles at regular centers and over slab joints.

REINFORCEMENT NOTES:

1. All workmanship and materials shall be in accordance with the requirements of the relevant current Australian Standard Codes (AS3600-2001 & AS2870-1996), Building By-Laws and the ordinances of the relevant Loca Authority.
2. Reinforcement is presented diagrammatically. It is not necessarily shown in true projection.
3. Provide the Engineer with 48 hours notice of reinforcement being ready for inspection.
4. No concrete is to be poured without the prior approval of the Engineer.
5. Splices in reinforcement shall be made only in the positions shown. Where not specified, laps shall be in accordance with AS3600 and not less than: N12 - 400, N16 - 500, N20 - 650, N24 -800, N28 - 1100, N32 - 1400, N36 - 1700.
6. Welding of reinforcement will not be permitted.
7. Reinforcement must not be continuous through construction joints.
8. Spacement of sufficient "stools" under main bottom and top reinforcement to allow adequate support in correct position during concreting (but not generally greater than 900mm spacings).
9. Supply and lay steel fabric in flat sheets at splices. Provide 225 minimum overlap of transverse wires to all edges of slab fabric laps. (ie two wires of each sheet overlapping plus 25mm)
10. Provide 'L' bars to all beam corners and intersections. 500 minimum laps.
11. Reinforcement Symbols:
    N Grade 500 MPa Deformed Bar
    SL Grade 500 MPa Deformed Slab Mesh
    Y Grade 410 MPa Deformed Bar
    S Grade 250 MPa Deformed Bar - Pool Construction
    R Grade 250 MPa Plain Bar

STRUCTURAL STEEL WORK NOTES:

1. All steel works is to be carried out in accordance with the current edition of:
   AS4100 - SAA steel structures code - AS1554
   SAA code for welding in buildings
   SA MA 1.8 - Fabrication
   SA MA 1.9 - Erection
   AISC - Standardized Structural Connection.
2. One full size paper copy of the shop detail drawings are to be submitted to the Engineer and approval of same obtained before commencing fabrication. Approval will not cover dimensional setout.
3. Unless noted otherwise, the size of all fillet welds is to be 6mm, SP category in accordance with AS1554. Welding electrode grade to be E48XX unless noted otherwise.
4. All bolts to be grade 8.8 high strength structural bolts to AS1252, snug tightened unless notes otherwise.
5. Internal steel work is to be thoroughly cleaned and coated with one coat of red oxide zinc chromate paint (or treated as specified) before erection.
6. Exposed steelwork, bolts, nut and washers to be hot dip galvanized. (HDG600 to AS/NZS 2312:2002
7. Where encased, steel work is to be wrapped in 10 gauge (3.25mm) black wire at 100mm pitch with 50mm cover.
8. The steel fabricator shall provide all bolts necessary for erection of the steel work and bolt holes and cleats necessary for the work as shown, noted or implied on these drawings or the architects drawings and specification.
9. The roof structure has been designed for normal roof loads only and does not allow for any extraneous loads such as hoists, monorails etc. except where shown on drawings.
10. When shop splices are necessary in beams or trusses, the position and details of the splice is to be approved by the Engineer.

TIMBER NOTES:

1. All timber design and construction shall be in accordance with AS1720 & AS1684 U.N.O.
2. All softwood shall be minimum stress grade F5 or MGP10 U.N.O.
3. All hardwood shall be minimum stress grade F14 U.N.O.
4. All laminated timber to be to G.L.T.A. specifications U.N.O.
5. External timber shall be either hardwood - Durability Class I or II, or impregnated grade F7 pine, pressure treated to AS1604 and re-dried prior to use. Supplementary treatment shall be applied to all cut surfaces. Details of preservative treatment shall be provided to the Engineer.
6. All bolts in timber construction shall be minimum M12 U.N.O and bolts, nuts and washers shall be hot dip galvanised. Bolt holes drilled in timber shall be exact size for bolts and spaced as documented. Where no specified bolts spacing's to be in accordance with AS1720 & As1684. Washers under bolt head and nuts shall be at least 2.5 times bolt diameter.
7. Bolting Schedule:

edge distances	spacing
M10 - 50mm	M10 - 50mm
M12 - 60mm	M12 - 60mm
M16 - 80mm	M16 70mm
M20 -100mm	M20 - 80mm

   All bolts to timber work to be grade 4.6/s u.n.o. (not applicable to masonry anchors & cast-in bolts)
   Bolts, nuts & washers to be hot dip galvanised.
   Bolt holes in steel plates shall provide a snug fit not greater than 0.5mm larger than bolt diameter.
   Washers to timber:
   M12 bolts.....50sq x 3 thick
   M16 bolts.....65sq x 5 thick
   M20 bolts.....75sq x 5 thick
   M24 bolts.....85sq x 6 thick (refer to detail drawings)
8. At the end of the defects liability period, all bolted connections throughout entire project must be re-tightened.
9. Timber dimensions on the finish width and thickness of timber to be:
   a. Seasoned Softwood +5.0, -0 mm
   b. Unseasoned Softwood +3.0, -3 mm
   c. Seasoned Hardwood +2.0, -0 mm
   d. Unseasoned Hardwood +3.0, -3 mm
10. All timber joints and notches shall be 100mm minimum away from loose knots, severe sloping grain gum veins or other minor defects.
11. All details not shown on the engineering drawings shall be in accordance with AS1684 - 1996 Residential Timber Framing Code and best practices.
12. Roof Trusses Timber trusses shall be supplied pre-cambered by an amount equal to the applied dead load deflection. A design certificate is to be provided prior to truss manufacture and signed by Truss Engineer (RPEQ). Once the trusses are installed the truss engineer shall provide a certificate indicating that the trusses have been installed as required. The certificate should also indicate and cover all bracing and tie-down req'd.

CONTOUR
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Registered ISO 9001:2008

PLAN DETAILS:

Designed:	SD
Drawn:	BS
Checked:	RM
Datum:	AHD
Date:	JAN 2019

PLAN AMENDMENTS:

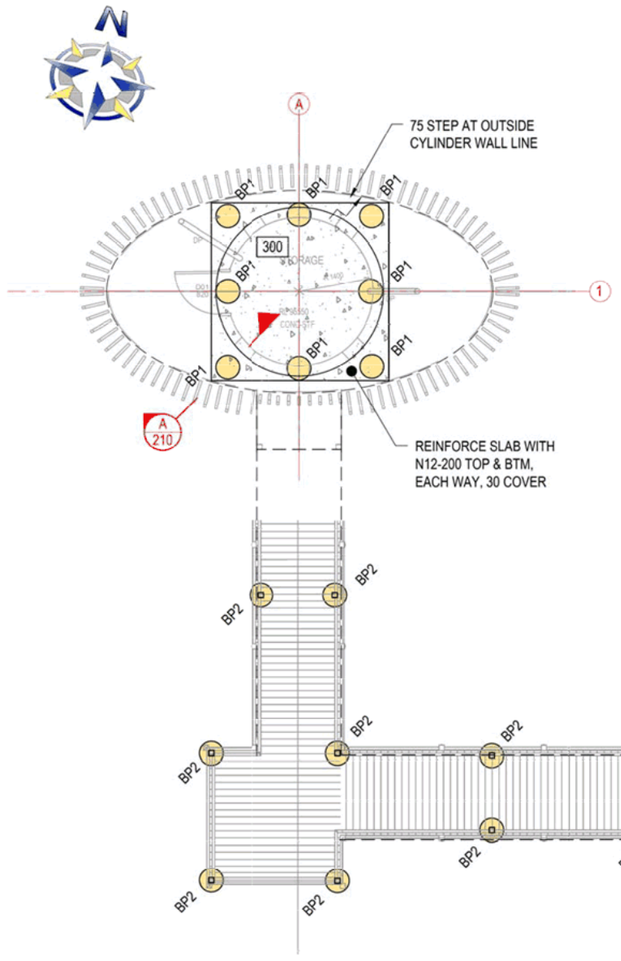
Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
GENERAL NOTES

PRELIMINARY ISSUE FOR REPORT ONLY

PLAN No. 5576 03 S 110 REVISION



FOOTING & SLAB PLAN
SCALE 1:100

DRAINAGE REQUIRMENTS

THE SITE HAS BEEN CLASSIFIED AS CLASS 'P/M' AND THE FOLLOWING DRAINAGE SYSTEMS NEED TO BE INCORPORA"ED INTO THE DESIGN.

1. SURFACE DRAINAGE TO DIRECT OVERLAND FLOW AND PREVENT PONDING AROUND BUILDING.
2. BASE OF TRENCHES TO SLOPE AWAY FROM BUILDING, TOP ON TRENCH TO BE BACKFILLED WITH CLAY AND WHERE PIPES PASS UNDER FOOTINGS BACKFILL FULL TRENCH TO PREVENT INGRESS OF WATER UNDER THE FOOTINGS.

SETOUT AND LEVELS

ALL SETOUT DIMENSIONS AND LEVELS ARE TO BE TAKEN FROM THE ARCHITECTURAL PLANS.
EXTERNAL SLABS TO FALL TO FIELD GULLIES OR FREE DRAINING SURFACES.

MEMBER SCHEDULE

FOOTINGS
BP1 450Ø x 2300D MIN BORED PIER AS DETAILED
BP2 450Ø x 2300D MIN BORED PIER AS DETAILED

JOINTS
SJ SAW OR TOOL JOINT AS DETAILED
CJ CONSTRUCTION JOINT - WITH DOWELS AS DETAILED
EJ EXPANSION JOINT - WITH DOWELS AS DETAILED

LEGEND
DENOTES EXTERNAL SLAB AS DETAILED

GENERAL NOTES

1. REFER SHEET S110 FOR GENERAL NOTES
2. REFER SHEET S210 FOR DETAILS

EXISTING SERVICES

BUILDING WORKS CONTRACTOR TO LOCATE AND PROTECT ALL EXISTING SERVICES PRIOR TO AND DURING SITE OPERATIONS.

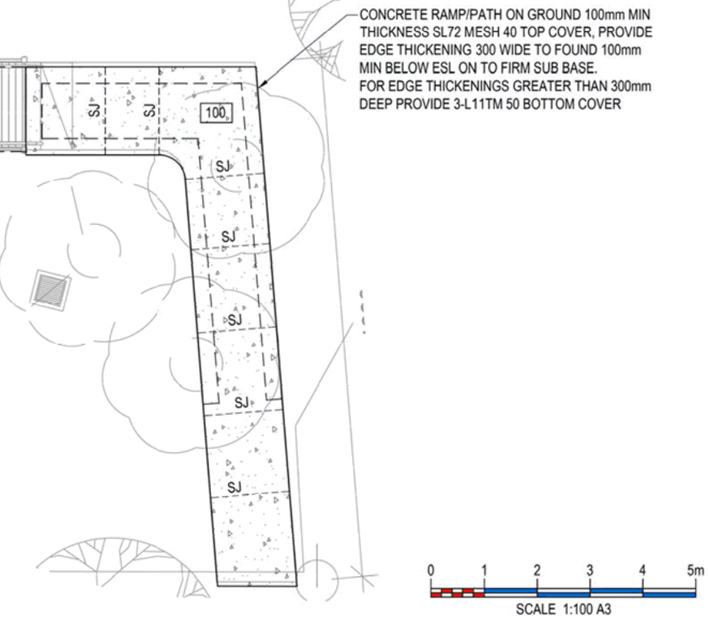
SITE EARTHWORKS

REFER SHEET S110 FOR GENERAL NOTES

FOUNDING NOTE

ALL LOAD BEARING FOOTINGS AND PIERS ARE TO FOUND INTO SIMILAR MATERIAL - 200 kPa MINIMUM ALLOWABLE BEARING CAPACITY, REFER GEOTECHNICAL REPORT. ALL SOFT SPOTS IN BASE OF FOOTING TRENCH TO REMOVED OR BACKHOE PIERS INSTALL AS DIRECTED BY SITE ENGINEER.

CONCRETE RAMP/PATH ON GROUND 100mm MIN THICKNESS SL72 MESH 40 TOP COVER, PROVIDE EDGE THICKENING 300 WIDE TO FOUND 100mm MIN BELOW ESL ON TO FIRM SUB BASE.
FOR EDGE THICKENINGS GREATER THAN 300mm DEEP PROVIDE 3-L11TM 50 BOTTOM COVER



SCALE 1:100 A3

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SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FOOTING & SLAB PLAN
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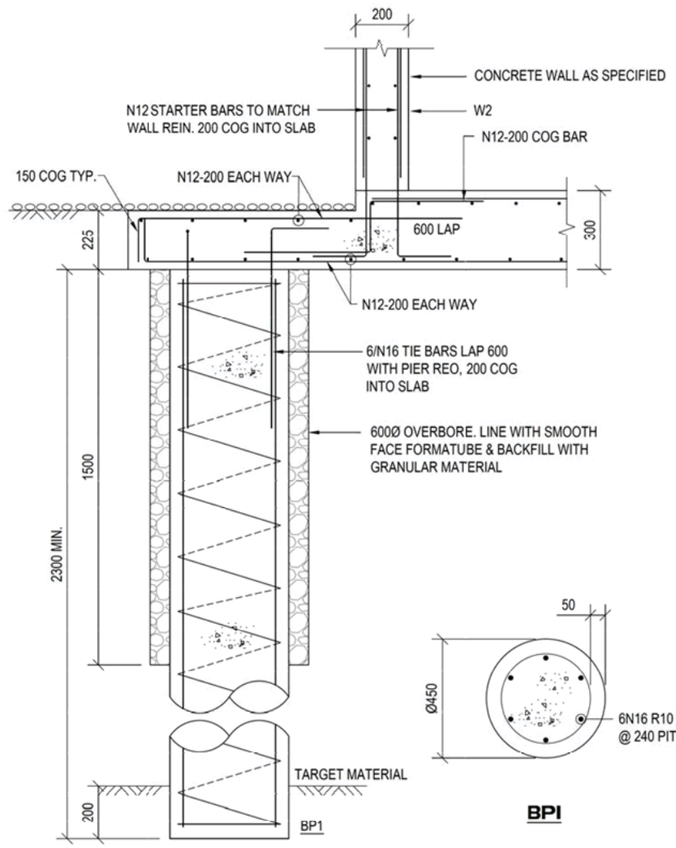
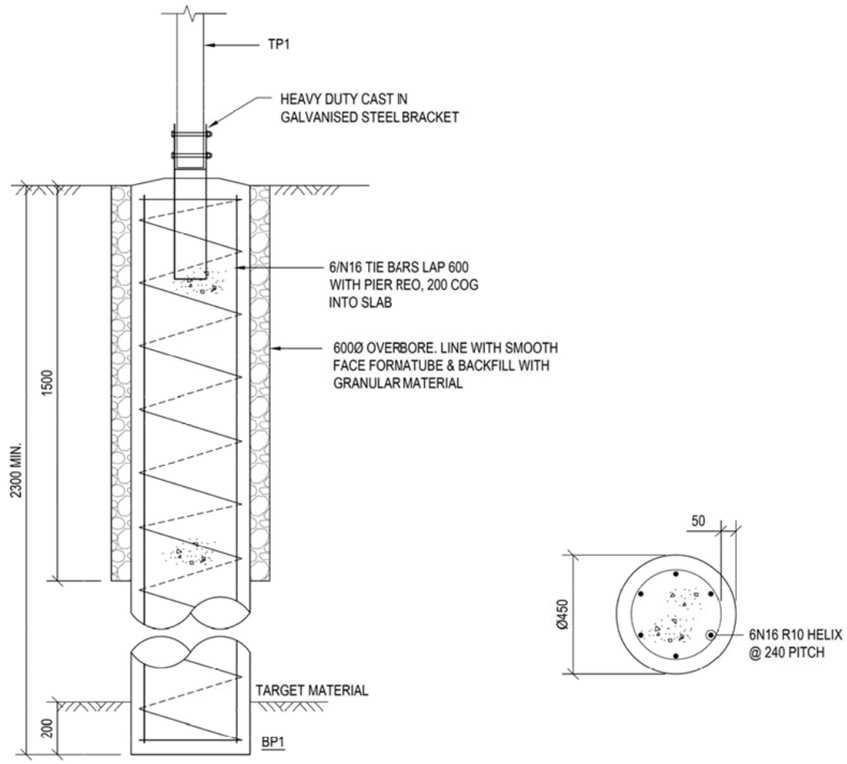
5576.03.S.200



BPI

SECTION A/200
SCALE 1:20

NOTE: PIERS TO FOUND MIN. 200 INTO HARD CLAYS OF ALLOWABLE BEARING CAPACITY 200Kpa



BP2

SECTION B/200
SCALE 1:20

REINFORCEMENT LAPS

N12	500
N16	650
N20	800
MESH	.....2 CROSS WIRES + 25MM

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
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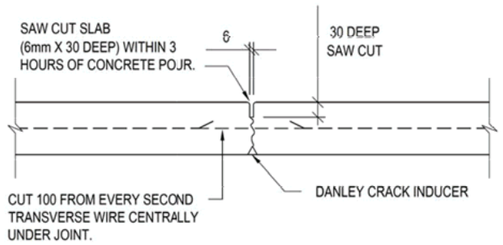
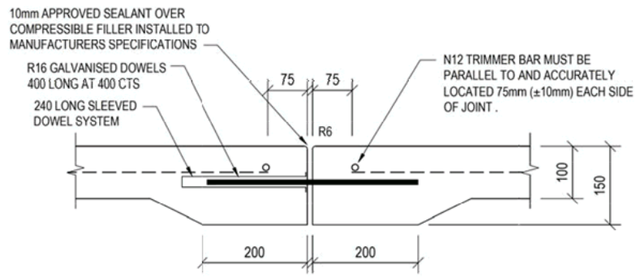
PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FOOTING & SLAB DETAILS
PRELIMINARY ISSUE FOR REPORT ONLY
5576.03.S.210

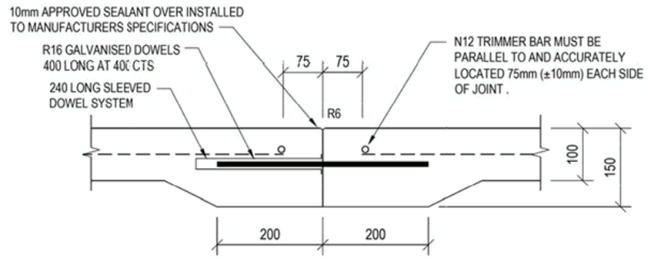


SAW OR TOOL JOINT
NOT TO SCALE



EXPANSION JOINT
NOT TO SCALE

NOTE
IT IS THE CONTRACTOR'S RESPONSIBILITY
TO MAINTAIN ALL BARS PERPENDICULAR
TO THE JOINT DURING CASTING



CONSTRUCTION JOINT
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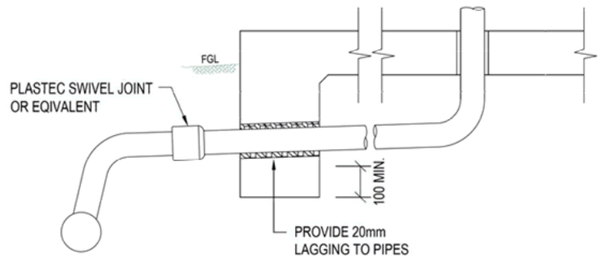
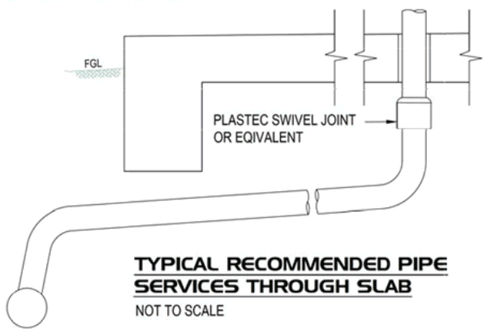
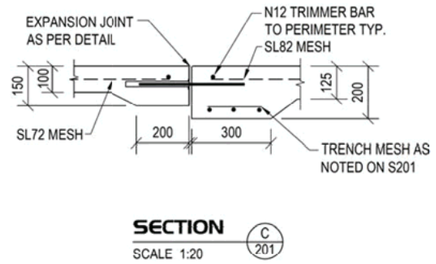


TYPICAL RECOMMENDED PIPE SERVICES THROUGH FOOTING
NOT TO SCALE

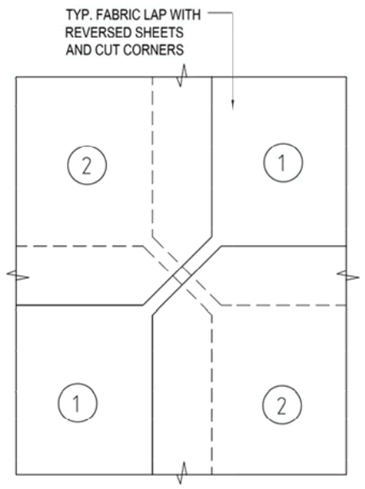
NOTE:
RECOMMENDATION IS MADE THAT THE
HOUSE SEWER LINE UTILSE 200mm
SLIPJOINTS BETWEEN JUNCTIONS



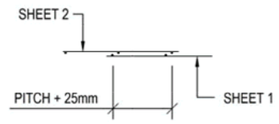
TYPICAL RECOMMENDED PIPE SERVICES THROUGH SLAB
NOT TO SCALE



SECTION C/201
SCALE 1:20



FABRIC LAP DETAILS
NOT TO SCALE



TYP. LAP ELEVATION

NOTE :
3 LAYERS OF WIRE
EVEN AT CORNERS

REINFORCEMENT LAPS

N12	500
N16	650
N20	800
MESH	.....2 CROSS WIRES + 25MM

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
PO Box 474, Buddina 4575, Qld
6 Innovation Parkway, Kawana Waters 4575, Qld
Ph. (07) 5493 9777 Fax (07) 5493 6888

PLAN DETAILS:

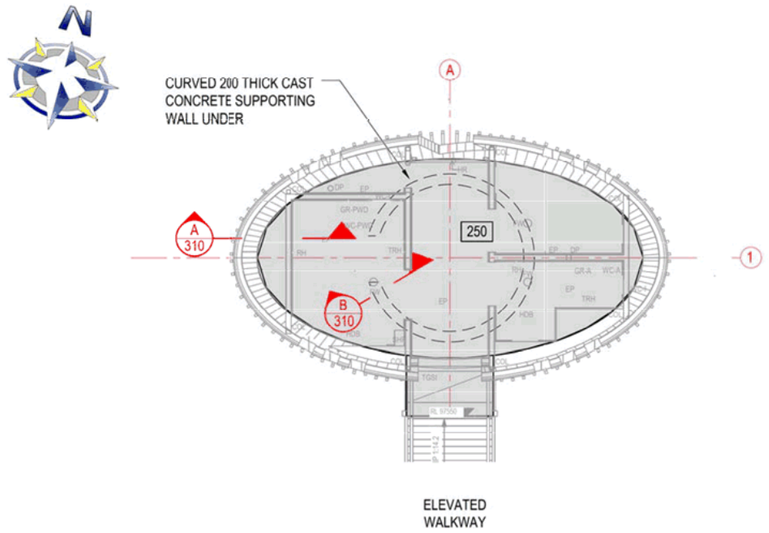
Designed:	SD
Drawn:	BS
Checked:	RM
Datum:	AHD
Date:	JAN 2019

PLAN AMENDMENTS:

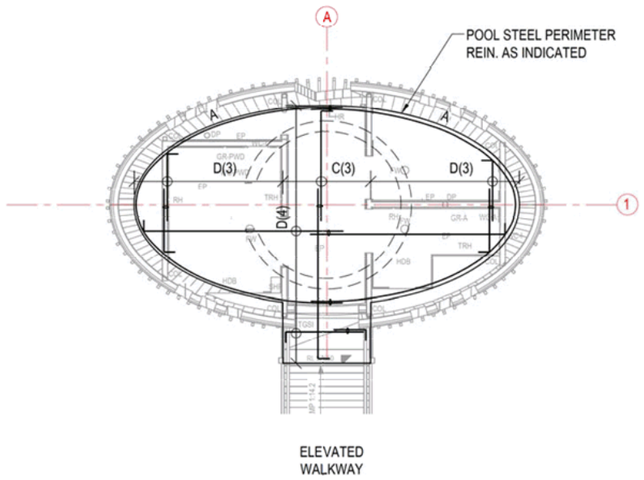
Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FOOTING & SLAB DETAILS
PRELIMINARY ISSUE FOR REPORT ONLY
5576.03.S.211



FIRST LEVEL SUSPENDED SLAB PLAN
SCALE 1:100



FIRST LEVEL TOP REINFORCEMENT PLAN
SCALE 1:100

MEMBER SCHEDULE

LEGEND

DENOTES REIN. CONCRETE WALL UNDER

DENOTES REIN. CONCRETE WALLS OVER

CONCRETE WALLS

W1 200 THICK WALL, N12-200 VERT & HORIZ. EACH FACE

REINFORCEMENT BAR LAYING SEQUENCE

BOTTOM REINFORCEMENT
(1) INDICATES LAY FIRST
(2) INDICATES LAY SECOND

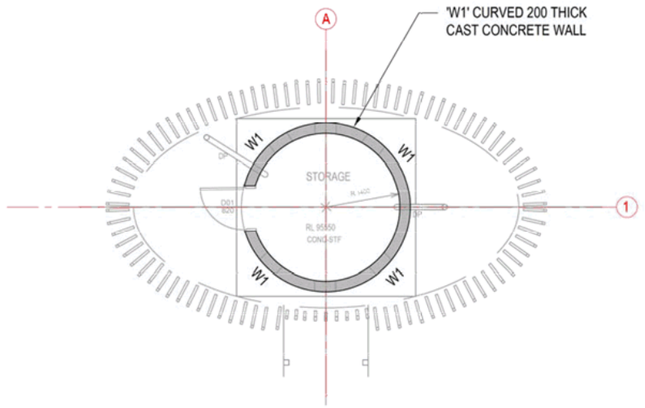
TOP REINFORCMENT
(3) INDICATES LAY THIRD
(4) INDICATES LAY FOURTH

REINFORCEMENT BAR SCHEDULE

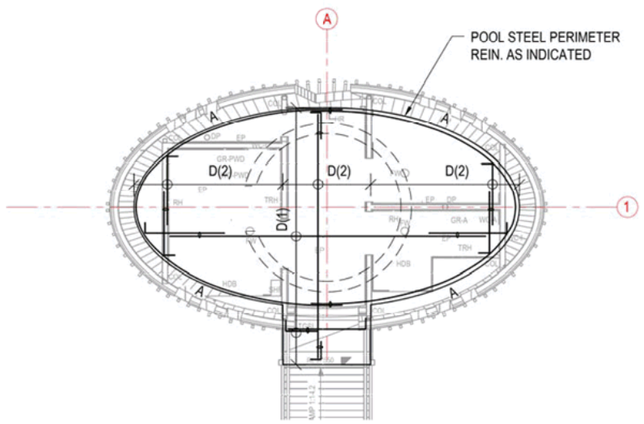
MARK	SIZE & CTRS
A	2/S12
B	1/S12
C	N12-200
D	N12-250
E	N12-300

SUSPENDED SLAB NOTES

1. REFER SHEET S110 FOR GENERAL NOTES
2. REFER SHEET S410 FOR DETAILS
3. REFER ARCHITECTURAL PLANS FOR CONFIRMATION OF ALL LEVELS, CLEARANCES, STEPS, FALLS AND SETOUT. REPORT ANY DISCREPANCIES FOR CLARIFICATION.



GROUND LEVEL WALL PLAN
SCALE 1:100



FIRST LEVEL BOTTOM REINFORCEMENT PLAN
SCALE 1:100

0 1 2 3 4 5m
SCALE 1:100 A3

CONTOUR
A.B.N. 62 117 061 659
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PLAN DETAILS:

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PLAN AMENDMENTS:

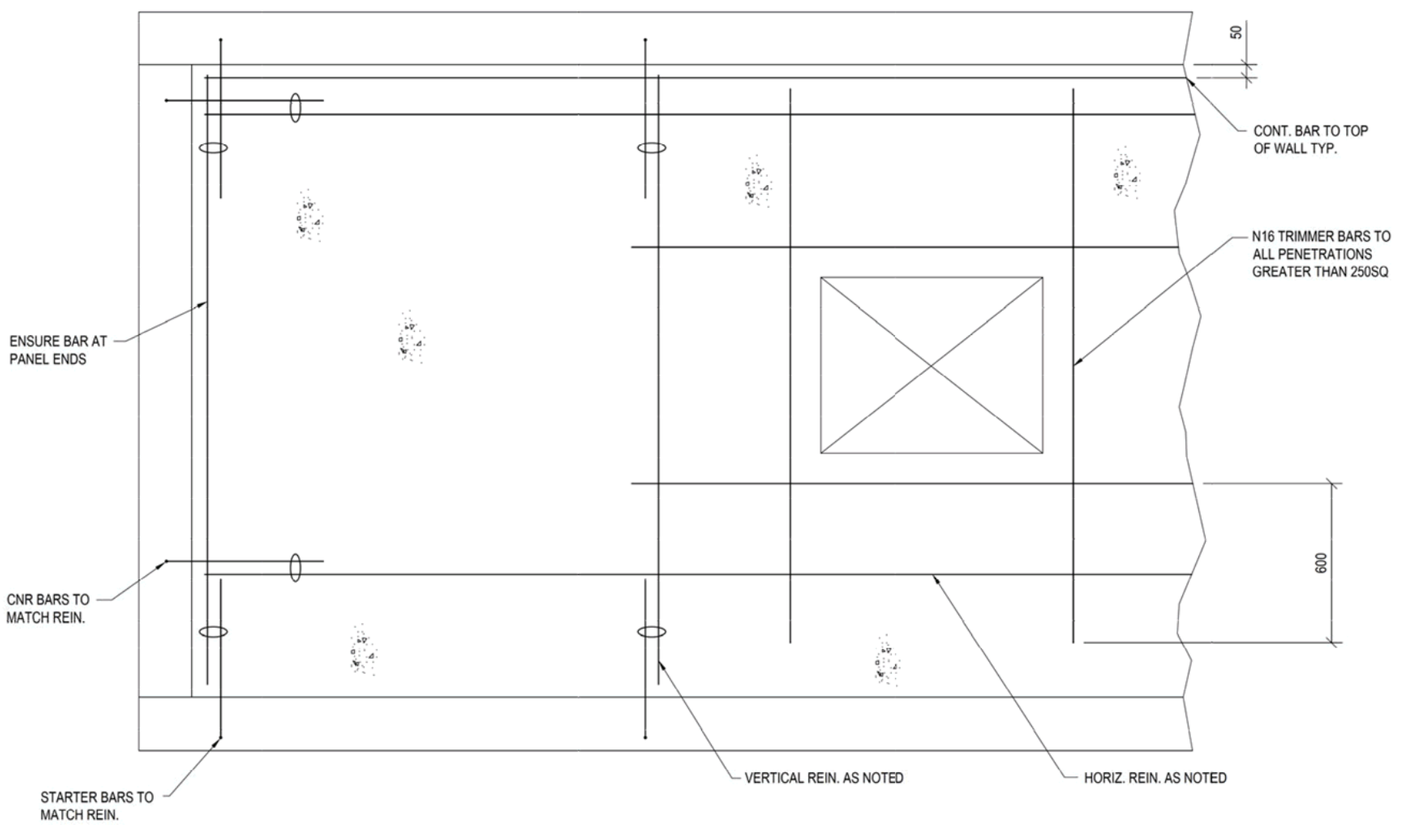
Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

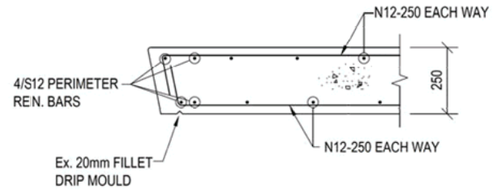
PLAN TITLE:
GROUND LEVEL WALL AND FIRST LEVEL SUSPENDED SLAB PLAN
PRELIMINARY ISSUE FOR REPORT ONLY

PLAN No: 5576.02.S.300

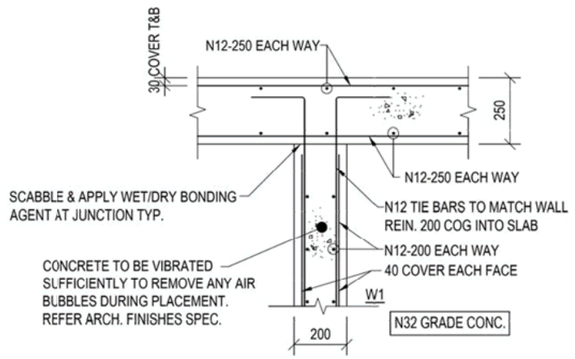
NOTE: COLD JOINTS BTWN SLAB AND RC WALL TO BE ADEQUATELY WATERPROOFED TO PREVENT WATER INGRESS



GENERAL METHOD OF WALL REINFORCEMENT
SCALE 1:20



SECTION A/300
SCALE 1:20



SECTION B/300
SCALE 1:20

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
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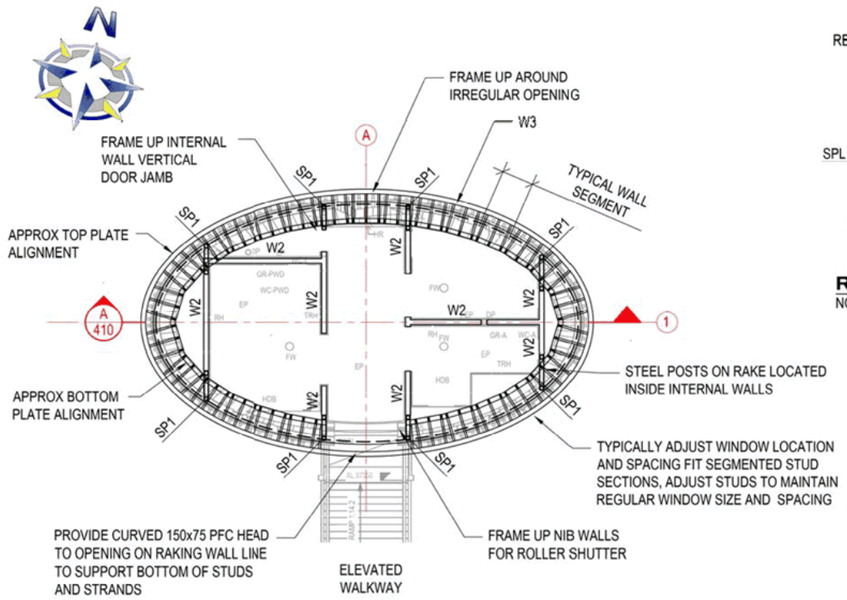
PLAN AMENDMENTS:

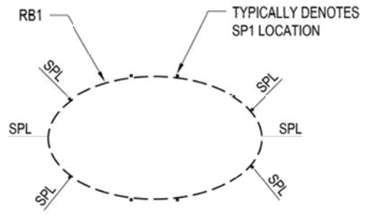
Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
SUSPENDED SLAB DETAILS
PRELIMINARY ISSUE FOR REPORT ONLY
5576 03 S 310



FIRST LEVEL WALL PLAN
SCALE 1:100



ROOF BEAM SPLICE LOCATION
NOT TO SCALE

WALL FRAMING SCHEDULE

W2 - INTERNAL STUD
TOP PLATE 2/90x45 MGP12
STUDS 90x45 MGP12 @450 CTS - 2.4m MAX
35x90 NOGGING @1350 CTS MAX
BTM PLATE 1/90x45 MGP12

W3 - SEGMENTED EXT WALL ON RAKE
TOP PLATE 2/90x45 MGP12
STUDS 90x45 MGP12 @300 CTS MAX
UPPER PARAPET 600mm MAX
LENGTH 3.9m, SPAN 2.7m MAX
LOWER PARAPET 600mm MAX
35x90 NOGGING @1350 CTS MAX
AND AS DETAILED
BTM PLATE 2/90x45 MGP12

W4 - CIRCULAR EXTERNAL WALL
TOP PLATE 92mm FLEXITRAXX
STUDS 90x45 MGP12 @300 CTS - 2.0 MAX
35x90 NOGGING @1350 CTS MAX
BTM PLATE 92mm FLEXITRAXX FULLY SUPPORTED

W5 - OVAL INTERNAL WALL
TOP PLATE 92mm FLEXITRAXX
STUDS 90x45 MGP12 @300 CTS 2.0 MAX
35x90 NOGGING @1350 CTS MAX
BTM PLATE 92mm FLEXITRAXX FULLY SUPPORTED

ALL PLATES ARE NOT TO BE TRENCHED
ALL STUDS ARE NOT TO BE NOTCHED

MEMBER SCHEDULE

WALLS
W2 90 STUD VERTICAL LOAD BEARING AND BRACING WALL 6mm HARDIBOARD TO 6.0kN/m
W3 90 STUD RAKING WALL
W4 90 STUD CIRCULAR EXTERNAL WALL
W5 90 STUD OVAL INTERNAL WALL

FRAMING
SP1 75 x 5.0 SHS POST ON RAKE
RB1 150 x 75 PFC CURVED ROOF BEAM
RJ1 150 x 42 LVL15 ROOF JOIST @900 CTS MAX
R1 90 x 45 MPG12 RAFTERS @600 CTS

ROOF BATTENS
75 x 38 F11, 70 x 45 MGP12 or LYSAGHT TS40 (0.55 BMT) @900 CTS

GENERAL NOTES

REFER SHEET S110 FOR GENERAL AND OTHER NOTES. CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE DURING CONSTRUCTION. ALL FRAMING TO BE IN ACCORDANCE WITH AS1684 - 2006 AND GENERALLY IN A TRADESMAN LIKE MANNER. ALL SCREW CONNECTIONS TO BE PRE-DRILLED.

SETOUT AND LEVELS

REFER ARCHITECT'S DRAWINGS FOR ALL LEVELS, CLEARANCES, STEPS, FALLS AND SETOUT.

TIMBER PROTECTION

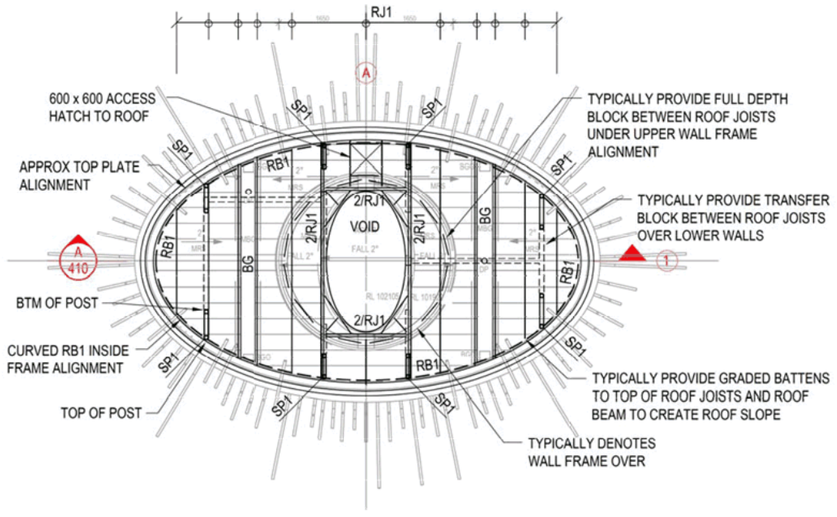
ENGINEERED TIMBER PRODUCTS ARE TO BE PROTECTED FROM THE WEATHER DURING CONSTRUCTION, THOSE PRODUCTS TO BE USED IN A PERMANENT EXTERNAL ENVIRONMENT SHALL BE END AND TOP CAPPED. ALL MEMBERS SHALL BE PRIMED AND PAINTED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS TO ENSURE CONTINUED SATISFACTORY PERFORMANCE OF THE MEMBER. REFER TO MANUFACTURERS SPECIFICATIONS FOR PARTICULAR DETAILS RELATING TO INDIVIDUAL PRODUCTS

GENERAL TIE DOWN

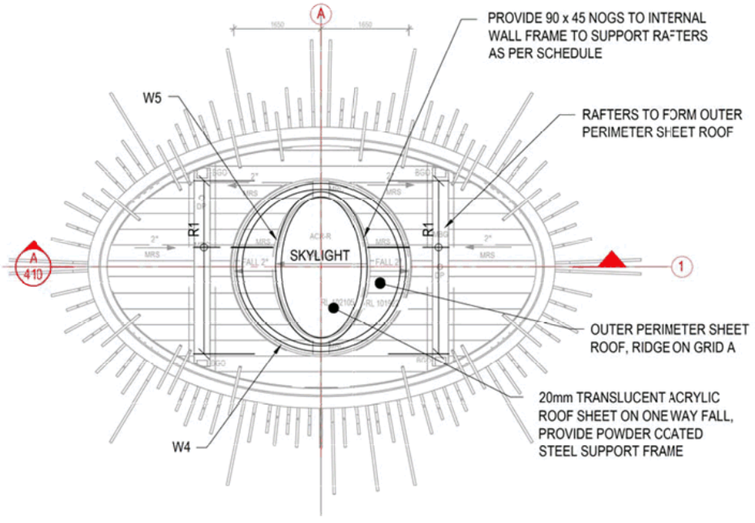
CONTRACTOR TO ENSURE ALL TIE DOWN TO BE CONTINUOUS TO SLAB ON GROUND.
REFER TO MANUFACTURERS SPECIFICATIONS FOR PARTICULAR PRODUCT DETAILS.

TIMBER CLASS

CLASS 3 AND 4 TIMBERS CLEAR OF THE GROUND AND EXPOSED TO THE WEATHER TO BE PRESERVATIVE TREATED TO MIN. LEVEL H3, WHERE THESE TIMBERS ARE IN CONTACT WITH THE GROUND PRESERVATIVE TREATMENT TO BE MIN. H5



FIRST LEVEL ROOF PLAN
SCALE 1:100



UPPER LEVEL WALL & ROOF PLAN
SCALE 1:100

0 1 2 3 4 5m
SCALE 1:100 A3

CONTOUR
A.B.N. 62 117 061 659
PO Box 474, Buddina 4575, Qld
6 Innovation Parkway, Kawana Waters 4575, Qld
Ph. (07) 5493 9777 Fax (07) 5493 6888

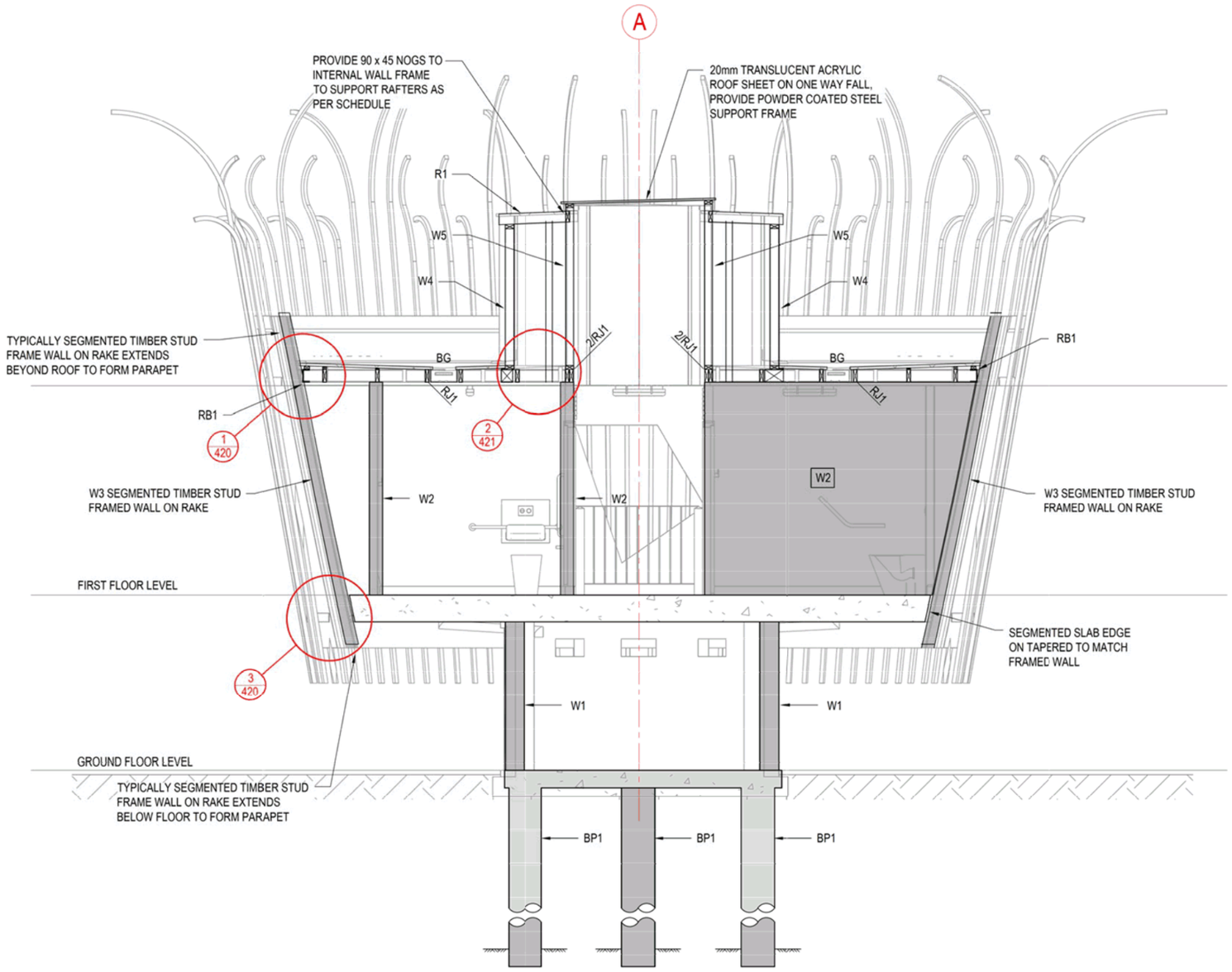
PLAN DETAILS:
Designed: SD
Drawn: BS
Checked: RM
Datum: AHD
Date: JAN 2019

PLAN AMENDMENTS:

Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FIRST AND UPPER LEVEL WALL AND ROOF FRAMING PLAN
PRELIMINARY ISSUE FOR REPORT ONLY



SECTION A/400
SCALE 1:50

0 0.5 1.0 1.5 2.0 2.5m
SCALE 1:50 A3

CONTOUR
A.B.N. 62 117 061 659
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6 Innovation Parkway, Kawana Waters 4575, Qld
Ph. (07) 5493 9777 Fax (07) 5493 6888
Registered ISO 9001:2008

PLAN DETAILS:
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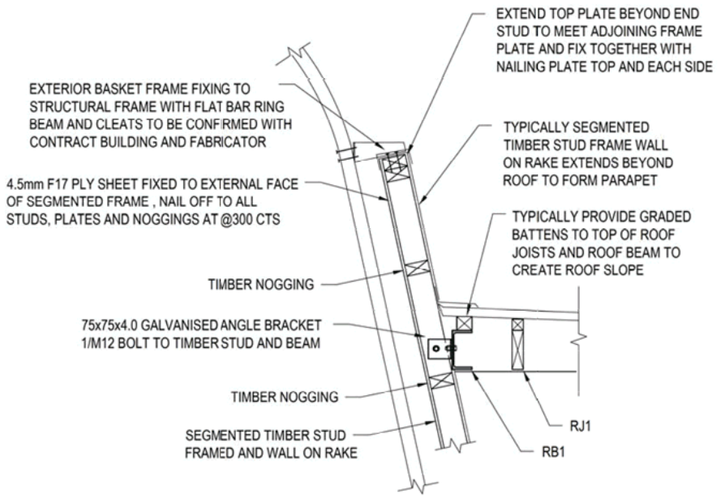
PLAN AMENDMENTS:

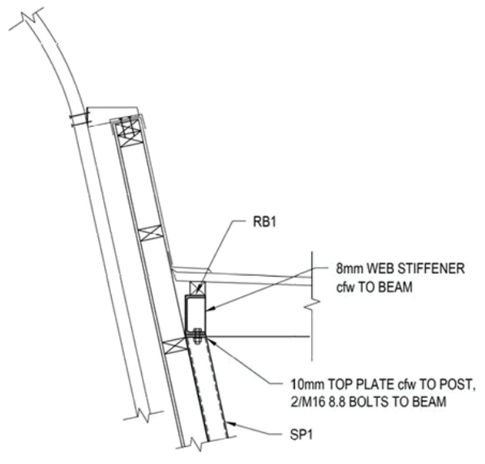
Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING ISAAC MOORE PARK, KENILWORTH SUNSHINE COAST REGIONAL COUNCIL

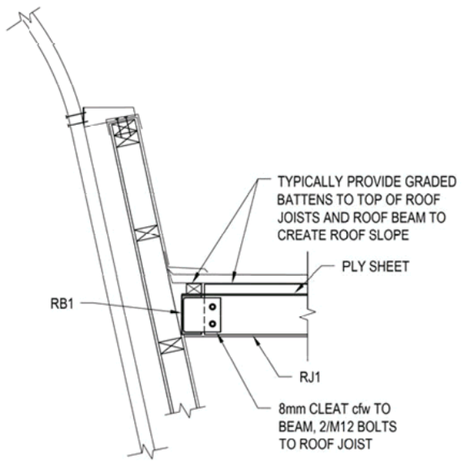
PLAN TITLE:
TYPICAL SECTION
PRELIMINARY ISSUE FOR REPORT ONLY
5576 03 S 410

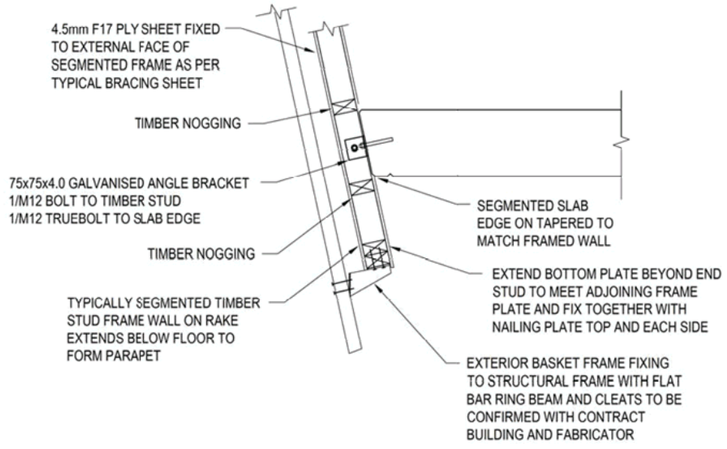


DETAIL 1/410
SCALE 1:20

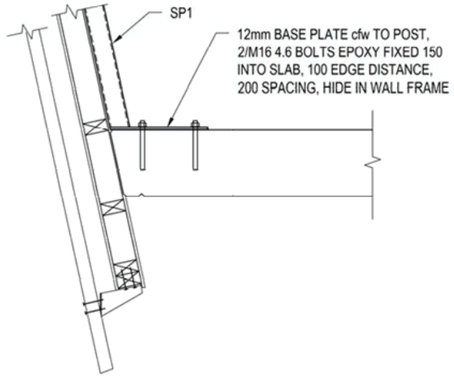


RBI TO SPI CONNECTION
SCALE 1:20





DETAIL 3/410
SCALE 1:20



SPI TO SLAB CONNECTION
SCALE 1:20

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
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6 Innovation Parkway, Kawana Waters 4575, Qld
Ph. (07) 5493 9777 Fax (07) 5493 6888

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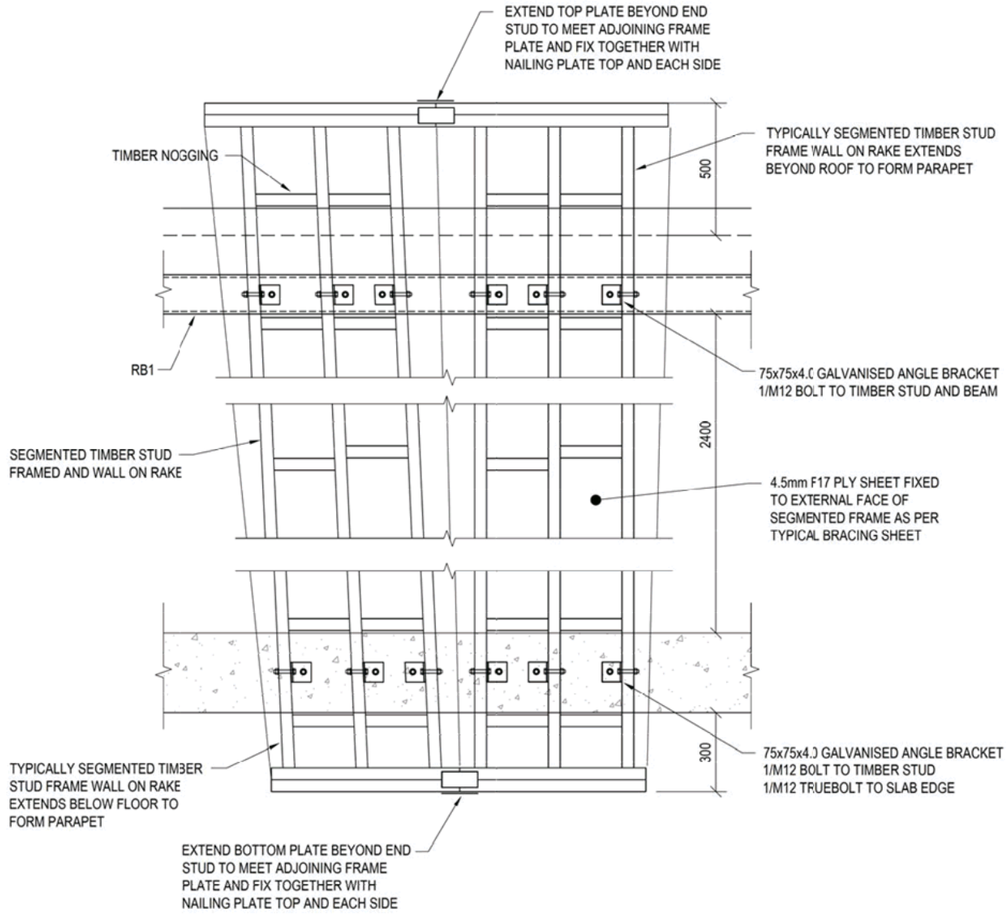
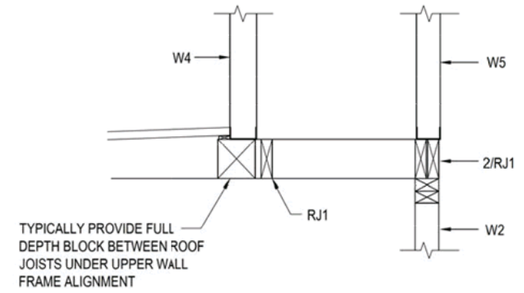
PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FRAMING DETAILS
PRELIMINARY ISSUE FOR REPORT ONLY
PLAN No. 5576.03.S.420



W3 - TYPICAL SEGMENTED WALL ELEVATION
SCALE 1:20



DETAIL 2/410
SCALE 1:20

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
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Ph. (07) 5493 9777 Fax (07) 5493 6888

PLAN DETAILS:	
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PLAN AMENDMENTS:

Rev	Date	Comment	Altered	Checked

PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FRAMING DETAILS
PRELIMINARY ISSUE FOR REPORT ONLY
PLAN No: 5576.03.S.421

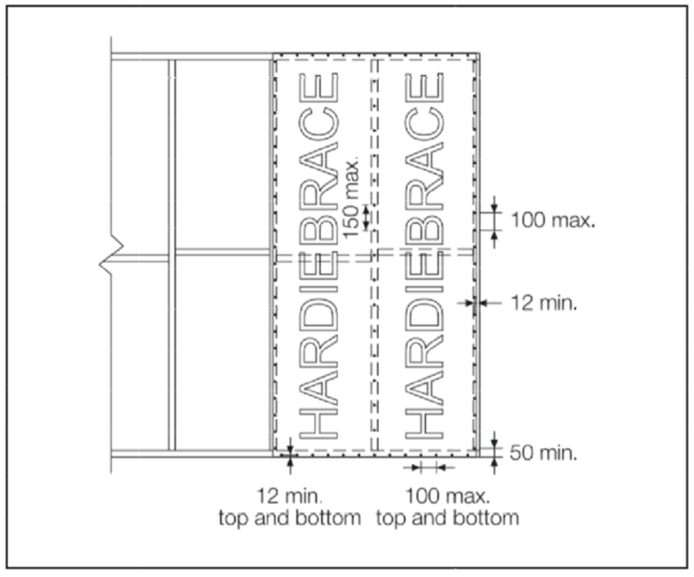
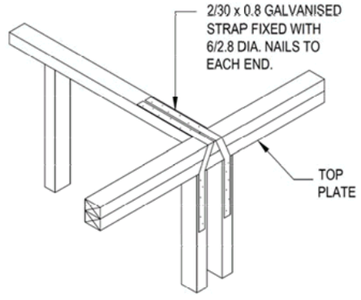


FIGURE 1 TYPE A NAILING DETAIL

TYPICAL BRACING WALL

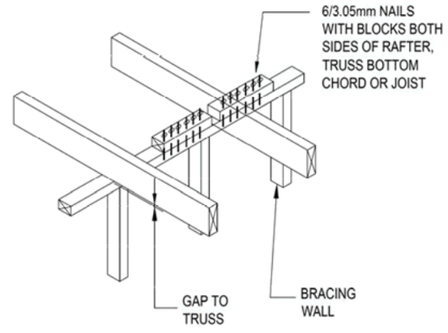
HARDIBRACE 6mm SHEET - 6.6kN/m
FIXING IN ACCORDANCE WITH MANUFACTURERS SPECIFICATION



SHEAR BLOCK S1 - 1 STRAP - CAPACITY 4.9KN

SHEAR BLOCK S2 - 2 STRAPS - CAPACITY 9.8 KN

(INTERNAL BRACING WALL TO EXTERNAL WALL)



SHEAR BLOCK S3 - CAPACITY 5.0 KN

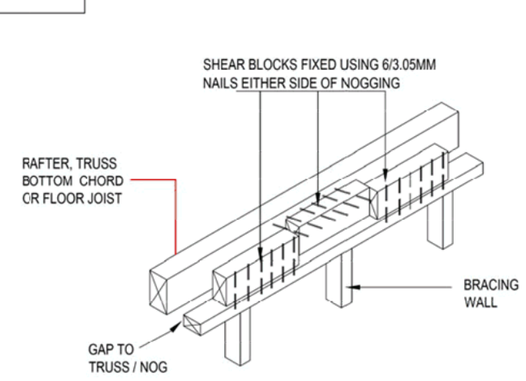
(BRACING WALL PERPENDICULAR TO TRUSSES, RAFTERS OR FLOOR JOISTS)

NAILING PLATE OR FRAMING ANCHOR (LEGS NOT BENT) TO BOTH ENDS OF NOGGING WITH 6/2.8MM DIA. NAILS EACH END. OR 2 / NO. 14 TYPE BATTERN SCREWS AT EACH END.

"POWERTRUSS" WALLS TO HAVE 2/NOGGINGS ON FLAT WITH 3/TYPE17 14-10 SCREWS EACH BLOCK TYP.

SHEAR BLOCK S5 - CAPACITY 5.0 KN

(BRACING WALL PARALLEL TO TRUSSES, RAFTERS, OR FLOOR JOISTS)



SHEAR BLOCK S6 - CAPACITY 5.0 KN

(BRACING WALL PARALLEL TO TRUSSES, RAFTERS, OR FLOOR JOISTS)

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
PO Box 474, Buddina 4575, Qld
6 Innovation Parkway, Kawana Waters 4575, Qld
Ph. (07) 5493 9777 Fax (07) 5493 6888

PLAN DETAILS:
Designed: SD
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PLAN AMENDMENTS:

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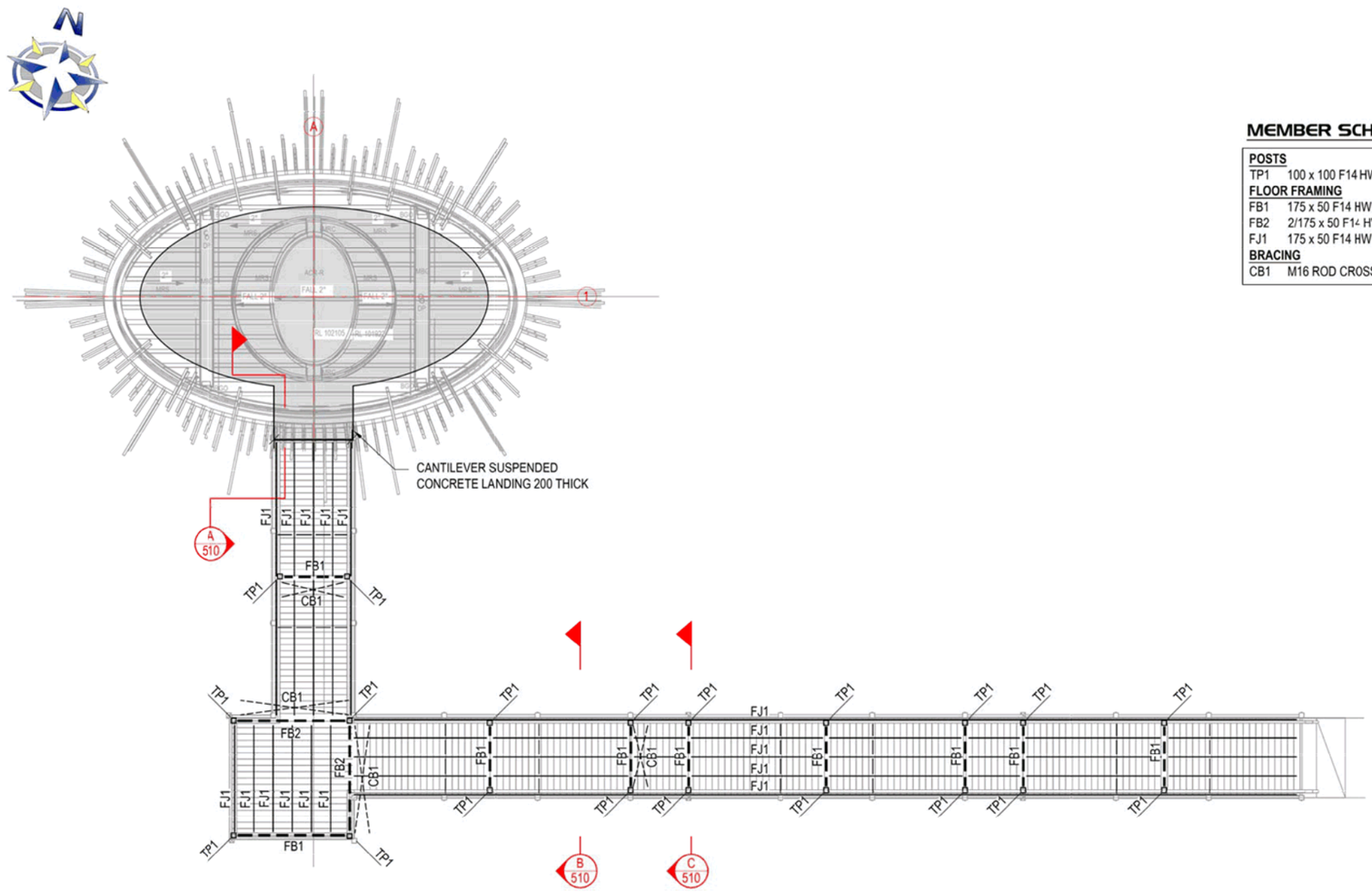
PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
FRAMING DETAILS

PRELIMINARY ISSUE FOR REPORT ONLY

PLAN No. 5576.03.S.433



MEMBER SCHEDULE

POSTS	
TP1	100 x 100 F14 HW TIMBER POST
FLOOR FRAMING	
FB1	175 x 50 F14 HW FLOOR BEAM
FB2	2/175 x 50 F14 HW FLOOR BEAM
FJ1	175 x 50 F14 HW FLOOR JOISNT @400 CTS
BRACING	
CB1	M16 ROD CROSS BRACE WITH TURNBUCKLE

ELEVATED WALKWAY FRAMING PLAN

SCALE 1:100

0 1 2 3 4 5m

SCALE 1:100 A3

CONTOUR
A.B.N. 62 117 061 659
PO Box 474, Buddina 4575, Qld
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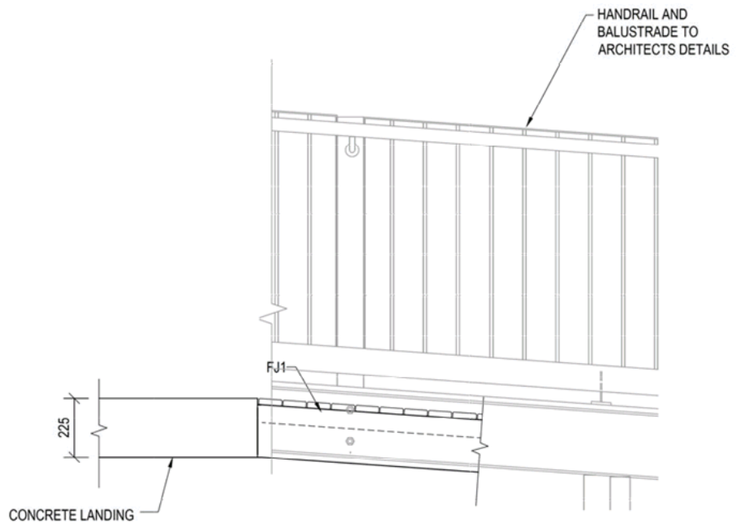
PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

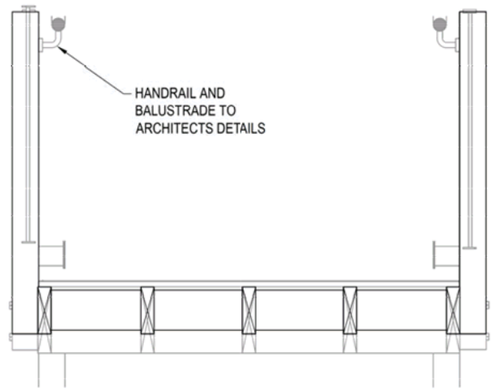
PLAN TITLE:
ELEVATED WALKWAY FRAMING PLAN

PRELIMINARY ISSUE FOR REPORT ONLY

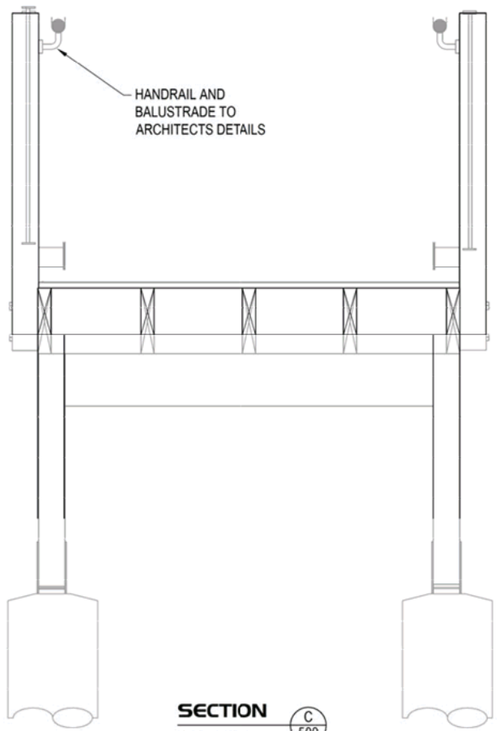
PLAN No. 5576.03.S.500 REVISION:



SECTION A/500
SCALE 1:20



SECTION B/500
SCALE 1:20



SECTION C/500
SCALE 1:20

0 0.2 0.4 0.6 0.8 1m
SCALE 1:20 A3

CONTOUR
A.B.N. 62 117 061 659
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PROPOSED AMENITIES BUILDING
ISAAC MOORE PARK, KENILWORTH
SUNSHINE COAST REGIONAL COUNCIL

PLAN TITLE:
WALKWAY FRAMING DETAILS
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